AMERICAN WOOD COUNCIL

November 2026

ERRATA

to the 2021 Edition of *Special Design Provisions for Wind & Seismic (SDPWS) with Commentary*

(All prior print and PDF versions)

Update 4.4.2 as shown in red strike-out below:

4.4.2 Wood Structural Panels Used in Walls Designed to Resist Only Uplift from Wind

~~Walls sheathed with wood structural panel sheathing or siding shall be permitted to be designed for simultaneously resisting shear and uplift from wind~~ Where walls sheathed with wood structural panel sheathing or siding are designed to resist only uplift from wind forces, they shall be in accordance with Section 4.4.1, except that panels with a minimum nominal panel thickness of 3/8" shall be permitted when installed with the strength axis parallel to the studs. The ASD allowable unit uplift shall be determined by dividing the tabulated nominal uplift capacity in Table 4.4.2 by the ASD reduction factor of 2.0. The LRFD factored uplift resistance shall be determined by multiplying the tabulated nominal unit uplift capacity in Table 4.4.2 by a resistance factor, ϕ_Z, of 0.65.

AMERICAN WOOD COUNCIL

August 2026

ERRATA

to the 2021 Edition of

Special Design Provisions for Wind & Seismic (SDPWS) with Commentary

(All prior PDF and print versions)

Correct equation C4.3.4-2 in the Commentary to remove the extra "+" as follows:

(bending, end post deformation excluding anchorage slip) *(shear, panel shear deformation and nail slip)* *(bending, wall anchorage deformation)*

$$\delta_{sw} = \frac{8vh^3}{EAb} + \frac{vh}{1000G_a} + \cancel{+}\frac{h}{b}\Delta_a \qquad \text{(C4.3.4-2)}$$

AMERICAN WOOD COUNCIL

May 2024

ERRATA

to the 2021 Edition of

Special Design Provisions for Wind & Seismic (SDPWS) with Commentary

(All prior print and PDF versions)

Page	Revision
48	Add footnote 6 in Table 4.4.1 as follows:

6. Tabulated nominal unit uplift capacities are applicable for carbon steel smooth shank nails of the specified type and size.

56 Replace "bs" with "b_s" in Appendix B.2(3a) as follows:

a. CLT panels forming either a single-panel or multi-panel wall shall have an aspect ratio, h/~~bs~~b_s , that is not less than the aspect ratio used for CLT shear wall panels in accordance with B.3.1 or B.3.7.

AMERICAN WOOD COUNCIL

April 2024

ERRATA

to the 2021 Edition of

Special Design Provisions for Wind & Seismic (SDPWS) with Commentary

(All prior print and PDF versions)

Page	Revision
13	Add the following superscript (shown in red underline) for Plywood Single Floor (under the column heading Wood Structural Panel Sheathing Type) in Table 3.2.2:

Plywood Single Floor(5)

33 Delete "s" in the following term in 4.3.5.4.1 (shown in red strike-out):

4.3.5.4.1 …. and the combined nominal unit shear capacity, $\nu_{n(sc)}$, …

91 Revise the following number in Example C4.2.6.1-2 (shown in red underline):

Δ_c = 0.054 in, joint deformation due to chord splice slip at each joint

AMERICAN WOOD COUNCIL

January 2023

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to the 2021 Edition of

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(All prior print and PDF versions)

Page **Revision**

33 Delete the following text (shown in red strikeout) for the terms $v_{n(1)}$ and $v_{n(2)}$ in 4.3.5.4.1:

$v_{n(1)}$ = nominal unit shear capacity for side 1, lbs/ft (from ~~Column A,~~ Tables 4.3A, 4.3B, 4.3C, or 4.3D)

$v_{n(2)}$ = nominal unit shear capacity for side 2, lbs/ft (from ~~Column A,~~ Tables 4.3A, 4.3B, 4.3C, or 4.3D)

Page **Revision**

41 Delete the following text (shown in red strikeout) in footnotes 1, 2, 3, 5, and 6 from Table 4.3B (footnotes 7 and 8 are not shown and remain the same):

1. Nominal unit shear capacities shall be adjusted in accordance with 4.1.4 to determine ASD allowable unit shear capacity and LRFD factored unit resistance. For general construction requirements see 4.3.6. For specific requirements, see 4.3.7.2 for wood structural panel shear walls over gypsum board. See Appendix A for common and box nail dimensions. ~~Nominal unit shear capacities are permitted to be increased to values shown for 15/32-inch (nominal) sheathing with same nailing provided (a) studs are spaced a maximum of 16 inches on center, or (b) panels are applied with long dimension across studs.~~
2. For species and grades of framing other than Douglas-Fir-Larch or Southern Pine, reduced nominal unit shear capacities shall be determined by multiplying the tabulated nominal unit shear capacity by the Specific Gravity Adjustment Factor = [1-(0.5-G)], where G = Specific Gravity of the framing lumber from the NDS (Table 12.3.3A). The Specific Gravity Adjustment Factor shall not be greater than 1. ~~Apparent shear stiffness values, G_a, are based on nail slip in framing with moisture content less than or equal to 19% at time of fabrication and panel stiffness values for shear walls constructed with either OSB or 3-ply plywood panels. When 4-ply or 5-ply plywood panels or composite panels are used, G_a values for plywood shall be permitted to be increased by 1.2.~~
3. Apparent shear stiffness values, G_a, are based on nail slip in framing with moisture content less than or equal to 19% at time of fabrication and panel stiffness values for shear walls constructed with either OSB or 3-ply plywood panels. When 4-ply or 5-ply plywood panels or composite panels are used, G_a values for plywood shall be permitted to be increased by 1.2. ~~Where panels are applied on both faces of a shear wall and nail spacing is less than 6″ on center on either side, panel joints shall be offset to fall on different framing members as shown below. Alternatively, the width of the nailed face of framing members shall be 3″ nominal or greater at adjoining panel edges and nails at all panel edges shall be staggered.~~
4. Where moisture content of the framing is greater than 19% at time of fabrication, G_a values shall be multiplied by 0.5.
5. Where panels are applied on both faces of a shear wall and nail spacing is less than 6″ on center on either side, panel joints shall be offset to fall on different framing members ~~as shown below~~. Alternatively, the width of the nailed face of framing members shall be 3″ nominal or greater at adjoining panel edges and nails at all panel edges shall be staggered.
6. Galvanized nails shall be hot-dipped or ~~tumbled~~ mechanically deposited.

AMERICAN WOOD COUNCIL

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ERRATA

to the 2021 Edition of

Special Design Provisions for Wind & Seismic (SDPWS) with Commentary

(PDF versions prior to 06-22-22 and printed version dated 12-21)

Page	**Revision**
45	Add the following text (shown in red underline) to the last sentence in 4.4.2:

The LRFD factored uplift resistance shall be determined by multiplying the tabulated nominal unit uplift capacity in Table 4.4.2 by a resistance factor, ϕ_z, of 0.65.